- [8] Ribbing, C.-G. and Roos, A., "Semi-Conductor vs. Metal Based Multilayer Coatings on Energy Conserving Windows," Scandinavian Symposium on Building Physics, Lund, 24-26 Aug. 1987.
- [9] Jonsson, B., "Heat Transfer Through Windows-During the Hours of Darkness with the Effect of Infiltration Ignored," Swedish Council for Building Research, Document D13:1985, Stockholm, 1985.
- [10] Blomsterberg, Å., "K-värde hos fönster (Window U-values)," Nordtest Symposium, Stockholm, 1981.

Martha G. Van Geem<sup>1</sup>

# Heat Transfer Characteristics of a Recently Developed Lightweight Structural Concrete

**REFERENCE:** Van Geem, M. G., "Heat Transfer Characteristics of a Recently Developed Lightweight Structural Concrete," Insulation Materials, Testing, and Applications, ASTM STP 1030, D. L. McElroy and J. F. Kimpflen, Eds., American Society for Testing and Materials, Philadelphia, 1990, pp. 437-463.

ABSTRACT: A lightweight structural concrete was developed for use in exterior walls of low-rise residential and commercial buildings. The lightweight concrete has a unit weight of 800 kg/m<sup>3</sup> (50 pcf), a compressive strength of 13.8 MPa (2000 psi), and a thermal conductivity of 0.23 W/m · K (1.6 Btu · in ./h · ft<sup>2</sup> · °F). Lightweight concretes have not been previously developed with this combination of low density and moderate strength. The most commonly used concrete, normal weight concrete, has a unit weight of approximately 2320 kg/m<sup>3</sup> (145 pcf), a compressive strength in the range of 17 to 41 MPa (2500 to 6000 psi), and a thermal conductivity of 1.7 to 2.3 W/m · K (12 to 16 Btu · in ./h · ft<sup>2 · °</sup>F).

The portland cement concrete developed for this project can be used to combine structural, thermal insulation, and heat storage capacity functions of exterior walls in one element. For many climates this concrete can be used without additional insulation as a complete wall system in lowrise buildings.

Heat transfer characteristics of two 200-mm (8-in.)-thick, full-size wall assemblies were evaluated using a calibrated hot box (ASTM C 976). One test specimen, designated Wall L, was a 200mm (8-in.)-thick wall constructed entirely of the newly developed lightweight structural concrete. The second specimen, designated Wall S, was the same as the first except for a 150-mm (6-in.)high normal weight concrete strip running horizontally across the wall at midheight. The horizontal strip simulates a floor slab extending through an exterior wall.

Overall thermal resistances of Walls L and S, respectively, are 0.92 and 0.83 m<sup>2</sup> · K/W (5.2 and 4.7 h · ft<sup>2</sup> · °F/Btu) at 24°C (75°F). Thermal resistance of Wall S is 11% less than that for Wall L.

Tests under dynamic temperature conditions provide a measure of thermal response for selected temperature ranges. Dynamic response includes heat storage capacity as well as heat transmission characteristics of the wall assembly. Results from a 24-h period, sol-air temperature cycle showed that heat storage capacity of the low density concrete delayed heat flow through the test specimen. Average thermal lag for the 200-mm (8-in.)-thick lightweight concrete wall was 6 h.

Thermal and physical properties of the lightweight concrete were also measured on small-scale specimens. Concrete thermal conductivity, thermal diffusivity, specific heat, compressive strength, flexural strength, splitting tensile strength, shear strength, modulus of elasticity, drying shrinkage, and freeze/thaw resistance were determined.

Laboratory test results provide information on the thermal and physical performance of the new lightweight concrete.

**KEY WORDS:** calibrated hot box, energy, heat transmission, lightweight concrete, structural concrete, thermal conductivity, thermal mass, thermal resistance

This paper summarizes results from a project to develop portland cement concrete with sufficient thermal resistance and strength properties to serve as an effective thermal break in building envelopes.

Senior Research Engineer, Fire/Thermal Technology Section, Construction Technology Laboratories, **isc.**, Skokie, IL 60077.

A thermal break is an exterior building element made of a material with a high thermal resistance used in place of a material with a lower thermal resistance to reduce energy losses through a building envelope. A thermal break may range in size from a small plastic nail used in place of a metal nail, to a large sheet of insulation used to prevent energy losses through a building foundation. The term "structural" used as an adjective to "thermal break" implies that the material has load bearing capabilities.

A concrete was developed with an air-dry unit weight of 800 kg/m<sup>3</sup> (50 pcf), a compressive strength of approximately 13.8 MPa (2000 psi), and a thermal conductivity of 0.23 W/m·K (1.6 Btu · in./h · ft<sup>2</sup> · °F). Although it is envisioned that concrete with these properties could be used for many building components, project emphasis is to evaluate the concrete for use in exterior walls for low-rise buildings.

The portland cement concrete developed for this project combines the structural, thermal insulation, and heat storage capacity functions of exterior walls in one element. For many climates this concrete can be used as a complete wall system in low-rise buildings without additional insulation.

#### **Objectives and Scope**

Project work is reported in Refs 1 to 3. Reference 1 is a feasibility study to identify uses for the proposed lightweight portland cement concrete in buildings. Reference 2 includes (1) selection of materials and mix designs for the lightweight portland cement and lightweight polymer concretes, (2) physical and thermal properties of candidate concretes, and (3) casting and surface finishing techniques for the most desirable mixes. Reference 3 describes heat transfer measurements of full-size wall assemblies constructed of the developed portland cement concrete.

Heat flow through two walls was measured in the calibrated hot box test facility (ASTM C 976) at Construction Technology Laboratories, Inc. (CTL). One test specimen, designated Wall L, was an 200-mm (8-in.)-thick wall constructed entirely of the newly developed lightweight structural concrete. The second specimen, designated Wall S, was the same as the first except for a 150-mm (6-in.)-high normal weight concrete strip running horizontally across the wall at midheight. The horizontal strip simulates a floor slab extending through an exterior wall.

Walls were tested for steady-state temperature conditions to obtain average heat transmission coefficients, including total thermal resistance  $(R_T)$  and thermal transmittance (U). A comparison of test results for the two walls shows the effect of the normal weight concrete strip.

Wall L, the homogeneous concrete wall, was also tested under dynamic temperature conditions. Dynamic tests provided a measure of thermal response for selected temperature ranges. A simulated sol-air dynamic cycle was selected to permit comparison of results with those obtained in previous investigations [4-6].

The program was conducted at Construction Technology Laboratories, Inc., (CTL). The project was sponsored jointly by the U.S. Department of Energy (DOE) Office of Buildings and Community Systems, and the Portland Cement Association. It is part of the Building Thermal Envelope Systems and Materials Program (BTESM), Energy Division, at Oak Ridge National Laboratory (ORNL).

## Background

Concrete developed for this program will have lower heat transmission than concrete commonly used for low-rise building construction. A wall with low heat transmission will conserve energy.

#### Types of Concrete

Concrete is available in a wide range of weights and strengths. Normal weight concrete utilizes sand and gravel aggregate and is most commonly used for construction of structural concrete members. Normal weight concretes have a unit weight of approximately 2320 kg/m<sup>3</sup> (145 pcf), and compressive strengths of approximately 17 to 41 MPa (2500 to 6000 psi) are common. High-strength normal weight concretes have been developed with strengths exceeding 100 MPa (15 000 psi). Measured thermal conductivities of normal weight concretes range from 1.4 to 2.9 W/m  $\cdot$  K (10 to 20 Btu  $\cdot$  in./h  $\cdot$  ft<sup>2</sup>  $\cdot$  °F).

Concretes in the 1440 to 2080 kg/m<sup>3</sup> (90 to 130 pcf) range are known as structural lightweight concretes. These concretes have compressive strengths in the range of 17.2 to over 62.1 MPa (2500 to over 9000 psi), depending on materials, mix design, and other factors. Lightweight concretes typically have thermal conductivities ranging from 0.6 to 1.9 W/m·K (4 to 13 Btu·in./h·ft<sup>2</sup>·°F).

While normal weight and structural lightweight concretes have more than adequate strength for the proposed use, their thermal properties may be inadequate for external walls.

Concretes weighing 800 kg/m<sup>3</sup> (50 pcf) or less are called *insulating concretes*. Current technology limits the compressive strengths of these concretes to about 4.1 MPa (600 psi) [7]. These concretes typically have thermal conductivities of 0.07 to 0.22 W/m  $\cdot$  K (0.5 to 1.5 Btu  $\cdot$  in./h  $\cdot$  ft<sup>2</sup>  $\cdot$  °F).

A second category of lightweight concretes is in the weight range of 800 to about 1440 kg/m<sup>3</sup> (50 to about 90 pcf). These are usually called *fill concretes*. Concretes in this weight range have not been widely used and their development has been somewhat neglected. This is because of generally poor strength-weight relationships available with these concretes. However, it is at the lower limit of this category, in the range of 720 to 800 kg/m<sup>3</sup> (45 to 55 pcf) concrete, that an effort has been made to develop concrete which will meet strength and thermal resistance requirements desirable for external walls in low-rise buildings. Concretes with unit weights of 720 to 800 kg/m<sup>3</sup> (45 to 55 pcf) have thermal conductivities of approximately 0.22 W/m · K (1.5 Btu · in./h · ft<sup>2</sup> · °F).

#### **Thermal Properties of Concrete**

Aggregates used to make concrete with a desired unit weight are available in a wide range of unit weights. Thermal conductivity of concrete is primarily dependent on its unit weight which is a function of the constituent aggregates used to make the concrete. To a lesser extent, thermal conductivity is dependent on the cement paste. Generally, concrete conductivity increases exponentially with unit weight. Concrete with a unit weight of 800 kg/m<sup>3</sup> (50 pcf) has a thermal conductivity of approximately 0.22 W/m · K (1.5 Btu · in./h · ft<sup>2</sup> · °F), while concrete with a unit weight of 2240 kg/m<sup>3</sup> (140 pcf) has a thermal conductivity of approximately 2.3 W/m · K (16 Btu · in./h · ft<sup>2</sup> · °F).

Heat flow through a homogeneous wall subjected to steady-state temperature conditions is linearly related to the thermal conductivity of the wall material and the temperature differential across the wall. For dynamic temperature conditions, heat flow is dependent on the storage capacity of the wall material in addition to its thermal conductivity.

Exterior building walls are seldom in a steady-state condition. Outdoor air temperatures and solar effects cause cyclic changes in outdoor surface temperatures.

A conditioned building with high mass walls will have less energy losses to the outdoor environment than an identical building with low mass walls of equivalent thermal resistance [I]. Energy savings are most significant for outdoor diurnal temperature cycles that cause reversals in heat flow through walls.

Optimally, the least heat will flow through a wall having high thermal resistance and high storage capacity. Heat transmission properties are more sensitive to changes in thermal resistance than to changes in storage capacity. The goal of this project is to develop a concrete with the highest thermal resistance and therefore the lowest unit weight. The concrete unit weight is limited by the need for sufficient structural capacity, because strength generally decreases with decreasing unit weight.

#### VAN GEEM ON LIGHTWEIGHT STRUCTURAL CONCRETE 441

## **Concrete Mix Development**

#### Portland Cement Concrete

Portland cement concrete consists, essentially, of portland cement, aggregates, and water. Relatively small quantities of other materials are frequently included to enhance certain properties which may be desirable for specific applications. Generally, aggregate is between 60 and 75% and cement, water, and air between 25 and 40% of the concrete volume. Since aggregate volume is so high, its specific gravity greatly influences the weight of the concrete. While cement has the highest specific gravity, it occupies a relatively small volume. Since cement is the strength-producing ingredient, the amount that it can be reduced is limited.

Based on the above, the investigative procedure consisted of locating the lightest available. aggregates capable of producing concrete having sufficient structural capacity. With these ag gregates, mixes had to be designed having the lowest cement contents (to lower weight) consistent with obtaining the required strength. Chemical and mineral admixtures were used to enhance the concrete's fresh properties and strength-to-weight relationship.<sup>2</sup>

Structural lightweight aggregates are available in all parts of the country. Many of these aggregates are capable of producing relatively high strength concrete in the weight range of 1400 to 1800 kg/m<sup>3</sup> (90 to 115 pcf).

The aggregates used in this investigation were limited to those known by the principal investigator, by previous experience, to be capable of producing lower weight concretes with adequate strength, or those found in a search for additional desirable aggregates. Acceptance of an aggregate or concrete mix design was based on compressive strength and unit weight. Other properties were not determined on those mixes that did not meet the strength-to-weight criteria.

#### **Preliminary** Mix Development

Concrete mixes were made using seven aggregates, singly or in combination (Table 1). The number of mixes made with each aggregate varied from one to twelve, depending on the aggregate's potential for meeting the weight and strength objectives. Aggregate combinations were used in many cases in an attempt to take advantage of desirable properties found in fine or coarse sizes of certain aggregates.

The last column in Table 1 shows the average strength-to-weight ratio for mixes made with different aggregate combinations. Mixes utilizing 3M Macrolite<sup>3</sup> had the highest strength-toweight ratio and had the best chance of meeting the program objectives. Therefore mixes were made with this aggregate to optimize the strength-to-weight relationship and to provide test specimens for further testing.

Macrolite Ceramic Spheres (Fig. 1) is a recently developed ceramic supplied by the 3M Company of St. Paul, Minnesota. According to the company, arrangements are being made to produce this material commercially. A unique feature of this aggregate is that it has a relatively low water absorption of less than 0.5%. Most low-absorption lightweight aggregates have absorptions ranging from 6 to 14%. The aggregate was supplied in two sizes; 12.7 to 4.75 mm (1/2 in to No. 4) and 4.75 mm to 300 µm (No. 4 to No. 50).4

Fillite, furnished by Fillite USA, Inc. of Huntington, West Virginia, is described as hollow alumina silica microspheres. The particles are similar in size and chemical composition to fin ash. However, they are hollow and have a much lower specific gravity than most fly ash. The

The strength-to-weight ratio is the ratio of the concrete's compressive strength to its unit weight. <sup>3</sup>Product names used in this paper may be trademarked.

Aggregate sizes are described by sieve opening sizes in accordance with ASTM Specification for Wire Cloth Sieves for Testing Purposes (E 11).

<b>ABLE 1</b> —Portland cement concrete unit weights and compressive	strengths.
--	------------

		Unit We kg/m (pcf)	ight.	Compressive MF (ps	e Strength, <sup>(4)</sup> Pa ii)	Strength- to-Weight Ratio <sup>(b)</sup> kPa/(kg/m <sup>3</sup> ,
Aggregate <sup>(1)</sup>	No. of Mixes	Fresh <sup>(2)</sup>	28 day <sup>(3)</sup>	7. day <sup>(5)</sup>	28 day <sup>(3)</sup>	(psi/pcf)
Project Objective		-	<b>900</b> ( 50)		10.3 (1500)	12.9 (30.0)
Tufflite	1	1 <b>299</b> (81.2)	<b>898</b> (56.1)	3.4 (490)	5.9 (850)	6.6 (15.2)
Tufflite & Fillite	1	1278 (79.9)	941 (58.8)	4.2 ( 610)	7.0 (1020)	7.4 (17.3)
Liapor	3	1005 (62.8)	898 (56.1)	5.4 (780)	8.0 (1160)	8.9 (20.7)
Liapor & Fillite	4	1064 (66.5)	962 (60.1)	8.2 (1190)	11.0 (1600)	11.4 (26.6)
Liapor, Fillite & PQ Microspheres	1	1010 (63,1)	896 (56.0)	8.5 (1230)	9.5 (1380)	10.6 (24.6)
Liapor, Fillite & 3 M Macrolite	2	896 (56.0)	829 (51.8)	10 <i>.1</i> (1550)	11.6 (1690)	14.0 (32.4)
Tufflite & Liapor	1	1107 (69.2)	1014 (63.4)	9.0 (1300)	11.0 (1590)	10.8 (25.1)
Livlite & Liapor	1	1117 (69.8)	997 (62.3)	8.1 (1170)	10.2 (1490)	10.2 (23.8)
Liviite	1	1275 (79.7)	1080 (67.5)	7.9 (1150)	15.0 (2170)	13.9 (32.1)
livlite & Fillite	1	1213 (75.8)	970 (60.6)	4.0 (590)	10.3 (1500)	10.6 (24.8)
teca	1	1062 (66.4)	941 (58.8)	4.1 ( 600)	6.8 (980)	7.2 (16.7)
3 Racrolite & Fillite	8	790 (49.4)	779 (48.7)	9.6 (1400)	12.3 (1780)	15.8 (36.6)

 Product names used in this paper may be trademarked.
 ASTM Designation: C 138, "Standard Test Method for Unit Weight, Yield, and Air Content (Gravimetric) of Concrete\*

(3) Measured on 100x200-mm (4x8-in.) and 150x300-mm (6x12-in.) cylinders moist-cured 7 days at  $(23+1.7^{\circ}C (73.4+3^{\circ}F) \text{ and 100% RH, and then air-dried at 23+1.7^{\circ}C (73.4+3^{\circ}F) and 50+5% RH for$ the remaining 21 days.

(4) ASTM Designation: C 39, "Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens"

3) Measured on 100x200-mm (4x8-in.) and 150x300-mm (6x12-in.) cylinders moist-cured at 23+1.7°C 73,4+3°F) and 100% RH for 7 days.

(6) Ratio of 28-day compressive strength to 28-day air-dry unit weight.

Filite size range was 30 to 300 µm. Fillite was used to provide a very fine lightweight material to these aggregates which were deficient in that size range.

Tufflite is a naturally occurring volcanic pumice aggregate. Livlite is an expanded clay aggreste. Liapor and Leca are expanded shale aggregates produced by grinding and pelletizing shales or clays and firing in a rotary kiln. Thus Liapor and Leca are spherical compared to Tufflite and Livlite which are irregularly shaped.

Most aggregates had relatively high water absorptions and were batched in a saturated conditon to avoid rapid stiffening during mixing. However, the 3M Macrolite had an extremely low absorption and was batched in a dry condition.



FIG. 1-3M Macrolite Ceramic Spheres.

#### Final Mix Design

The final mix design is shown in Table 2. It was used for determining various concrete physical and thermal properties and for casting two full-size wall panels, designated Walls L and S, for determination of thermal properties. The same volumetric mix design was used for both panels. However, aggregate weights varied because of the differences in specific gravities of aggregates from different shipments. The amount of vinsol resin air entraining agent was varied slightly to obtain a unit weight of about 800 kg/m<sup>3</sup> (50 pcf).

#### **Physical and Thermal Properties of Small-Scale Specimens**

Selected physical and thermal properties were measured on specimens cast from six concrete mixes using 3M Macrolite as aggregates. The six mixes are similar to those presented in Table 2. Test results are summarized in Table 3. Reference 2 gives details of specimen preparations and test procedures.

Reference 2 also compares properties of the newly developed concrete to properties of conventional normal weight and lightweight concretes.

#### **Full-Size Test Specimens**

Two lightweight structural concrete walls were constructed by CTL and subsequently tested in a calibrated hot box. Walls were cast horizontally and have overall nominal dimensions of 2.62 by 2.62 m (103 by 103 in.).

#### Wall Construction

Wall L is a lightweight structural concrete wall with an average thickness of 203 mm (8.00 in.). Wall S is similar to Wall L except for a 150-mm (6-in.)-high normal weight concrete strip running horizontally across the wall at midheight. Average thickness of Wall S is 206 mm (8.13 in.).

#### VAN GEEM ON LIGHTWEIGHT STRUCTURAL CONCRETE 443

TABLE 2-Final portland cement concrete mix design.

	(	Quantities Quantities p	per 1.0 m <sup>3</sup> er 1.0 cu yd)
Material	Absolute Volume. m <sup>3</sup> (cu ft)	He (	ight, kg lb)
		Wall L	Wall S
ortland Cement	0.080 (2.16)	252 (425)	252 (425)
illica Fume	0.012 (0.33)	26.1 (43)	26.1 (43)
later	0.149 (4.01)	1 <b>49</b> (250)	1 <b>49</b> (250)
Air Content	0.060* (1.62)		
3M Macrollte 12.7 to 4.75 mm (1/2 <sup>m</sup> to <b>#</b> 4)	0.342 (9.25)	174 (293)	195 (327)
4.75 to 0.30 mm (#4 to #50)	0.329 (8.88)	273 (459)	277 (466)
1111te	0.028 (0.76)	20 (33)	20 (33)
finsol Resin, 2% Solution	1670-1950 ml (1275-1488 ml)	1.7 (2.81)	2.0 (3.28)
RDA, 4.55 ml/kg cement (7 oz/100 lb)	1160 ml (888 ml)	1.2 (1.96)	1.2 (1.96)
•			1

\*Air content estimated at 6%.

The concrete mix for Walls L and S are presented in Table 2. Reinforcement representative of actual wall construction was placed within Walls L and S. Reinforcement consisted of a single layer of 13-mm (No. 4) bars spaced 305 mm (12 in.) center-to-center in each direction. The reinforcement was located at the walls' approximate midthickness.

Threaded concrete inserts were cast into the walls at midthickness to aid in transporting walls after concrete had attained the necessary strength.

Walls L and S were allowed to cure in the formwork for approximately two weeks. After removing from formwork, Wall L was allowed to air dry in the laboratory at a temperature of  $18 \pm 6^{\circ}C$  (65  $\pm 10^{\circ}F$ ) for approximately three months. Wall S was air dried in the laboratory at a temperature of  $21 \pm 6^{\circ}C$  (70  $\pm 10^{\circ}F$ ) for approximately four months.

Before testing, the faces of Walls L and S were coated with a cementitious waterproofing material to seal minor surface imperfections. A textured, noncementitious paint was subsequently used as a finish coat. These coatings provided a white, uniform surface for both faces of each wall. Wall edges were left uncoated.

Measured weights, thicknesses, surface areas, and estimated moisture contents of Walls L and S are summarized in Table 4. Wall weights immediately before and after calibrated hot box tests are presented.

Reference 3 more fully describes wall construction.

TABLE 3-Physical and thermal properties of hardened concrete.

	•		
Property	Test Method	Pre-Test Curing	Measured Value
Unit Weight 28-day	ASTM: C567, as applicable	7 days 100% RH, 21 days 50±5% RH	793 kg/cu m (49.5 pcf)
Compressive Strength	ASTM: C39		
7-day		7 days 100% RH	11.5 MPa (1670 psi)
28-day		7 days 100% RH, 21 days 50±5% RH	13.8 MPa (2000 psi)
Splitting Tensile Strength	ASTM: C496		
7-day		7 days 100% RH	1.3 MPa (185 pel)
28-day		7 days 100% RH, 21 days 50±5% RH	0.9 MPa (135 psi)
Modulus of Rupture Flexural Strength)	ASTM: C78		
7-day		7 days 100% RH	1.8 MPa (260 pai)
28-day		7 days 100% RH, 21 days 5015% RH	1.7 MPa (250 pel)
Shear Strength	See Reference 2		
7-day		7 days 100% RH	2.0 MPs (290 pai)
28-day		7 days 100% RH, 21 days 5015% RH	1.8 MPa (260 pai)
Andrakas of Figurations	ASTM: CARO		
28-day		7 days 100% RH, 21 days 50±5% RH	6400 MPa (0.93x10^6 pai)
reezing and Thewing	ASTM: C666,	7 days 100% RH, 21 days 50±5% RH,	55% after 300 cycles
lesistance,	Procedure A	24 hrs soaked in water	
Native dynamec hodulus of elasticity	(treazing in water)		
	See Reference 2. (freezing in air after 1/2 hr water soak)	14 days 100% RH, 14 days 50±5% RH	123% after 150 cycles
orying Shrinkage	See Reference 2	7 days 100% RH, then 50±5% RH	
@P 161 days			0.088%
@ 179 days			0.087%
@ 355 Gays			0.093%
hermal Conductivity	ASTM: C177* @ 75°F	7 days 100% RH, 58 to 70 days 45±15% RH, then ovendry	0.23 W/m•K (1.61 Btu•in_/hr•sq ft•*F)
pecific Heat	US Army Corps of Engineers CRD-C124-73 (Ref. 8)	100% RH	
Saturated Surface Dry Air Dry			1060 J/kg+K (0.25 Btu/lb+*F) 460 J/kg+K (0.11 Btu/lb+*F)
hermal Diffusivity	US Army Corps of Engineers	100% RH	0.00096 sq m/hr
,	CHU-C36-73 (Hef. 8)		(0.0 (04 #4 (0(0))

\* Thermocouples for measuring specimen surface temperatures embedded flush with specimen surface.

#### Instrumentation

Eighty 20-gage, Type T thermocouples, corresponding to ASTM Temperature-Electromotive Force (EMF) Tables for Standardized Thermocouples (E 230), were used to measure temperatures during thermal testing. For each test wall, 16 thermocouples were located in the air space on each side of the test specimen, 16 on each face of the test wall, and 16 at the approximate concrete midthickness. The 16 thermocouples in each plane were spaced 525 mm (20<sup>3</sup>/s in.) apart in a  $4 \times 4$  grid over the wall area (Figs. 2 and 3).

An additional four thermocouples were located on each wall surface and at concrete midthickness along the centerline of the normal weight concrete strip of Wall S (Fig. 3).

## VAN GEEM ON LIGHTWEIGHT STRUCTURAL CONCRETE 445

TABLE 4-Summary of physical properties for Walls L and S.

	Measure	d Value
Property	Wall L	Wall S
Weight of Wall, kg (lb) Before testing	1250 (2760)	1320 (2910)
After testing	1240 (2720)	1310 (2890)
Unit Weight of Wall,* kg/m <sup>2</sup> (lb/ft <sup>2</sup> )	182 (37.4)	***
Unit Weight of Wall,* kg/m <sup>3</sup> (lb/ft <sup>3</sup> )	898 (56.0)	***
Average Wall Thickness, mm (in.)	203 (8.00)	207 (8.13)
Wall Area, m <sup>2</sup> (ft <sup>2</sup> ,)	6.86 (73.88)	6.87 (73.92)
Estimated Moisture Content**, % ovendry weight	2	2

\*Before calibrated hot box tests.

\*\*Estimated from air dry and ovendry weights of thermal conductivity specimens.

\*\*\*Not calculated because Wall S is not homogeneous.







FIG. 3-Wall S air. surface, and internal thermocouple locations.

Thermocouples measuring temperatures in the air space of each chamber of the calibrated hot box were located approximately 75 mm (3 in.) from the face of the test wall.

Surface thermocouples were securely attached to the wall with duct tape for a length of approximately 100 mm (4 in.). The tape covering the sensors was painted the same color as the test wall surface.

During wall construction, internal thermocouples were placed at wall midthickness on top of the first 100-mm (4-in.) concrete layer. To secure their location, thermocouples were taped to reinforcement or suspended by wire between reinforcement. The thermocouple junction was not placed in contact with the reinforcement. This was done for all internal thermocouples to avoid any influence by internal heat flow through reinforcement. Thermocouples were wired to form a thermopile such that an electrical average of four thermocouple junctions, located along a horizontal line across the grid, was obtained. Wires for internal thermocouples were routed through side formwork before casting the second 100-mm (4-in.) concrete layer.

One heat flux transducer measuring 100 by 100 mm (4 by 4 in.) was mounted on each of the indoor and outdoor surfaces of the test walls. Sensors were located near the center of the walls (Figs. 2 and 3). The surface of the heat flux transducer in contact with a wall surface was coated with a thin layer of high-conductivity silicon grease. The silicon grease provided uniform contact between the heat flux transducer and wall surface. Duct tape was used to secure heat flux transducers to the wall surfaces. The duct tape was painted the same color as the test wall surface. Heat flux transducers were calibrated using results from steady-state calibrated hot box tests on Wall L.

#### **Calibrated Hot Box Test Facility**

Heat flow through Walls L and S was measured under steady-state and dynamic temperature conditions. Tests were conducted in the calibrated hot box facility shown in Figs. 4 and 5. Tests



FIG. 4-Calibrated hot box test facility.



FIG. 5-Schematic of calibrated hot box.

Metering Climatic Max., Min., Chamber, Chamber, (°C) (°C) % 7 7 35 20 24 23 (75) (74)	Metering Chamber, %         Climatic Chamber, %         Max., (°C)         Min., (°C)           35         20         24         23           35         20         24         23           (75)         (74)         24         22           34         17         24         22           (75)         (72)         (72)	Melering Chamber, %         Climatic Chamber, %         Max., (°C) %         Min., (°C) %           35         20         24         23           35         20         24         23           34         17         24         22           33         11         23         22           (74)         (74)         (72)	Metering Chamber, %       Climatic Chamber, %       Max., (°C)       Min., (°C)         35       20       24       23         35       20       24       23         (75)       (74)       (74)         34       17       24       22         33       11       23       22         33       10       24       23         55       21       23       (74)         50       10       24       23         (74)       (72)       33       10	ith ASTM Test for Thermal Performance of Building Assem- for Box (C 976). a brief description of the calibrated hot box. Instrumentation bed in Refs 3 and 9. hlv insulated chambers (Fig. S) The walls ceiling and floor
24 35 20 24 23 38) (75) (74)	35         20         24         23           35         20         24         23           (75)         (74)         24         22           34         17         24         22           (75)         (75)         (72)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	•
	34 17 24 22 (75) (72)	34         17         24         22           (75)         (72)         (72)           33         11         23         22           (74)         (72)         (72)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	r lin., °C) F
9     33     11     23     22       9)     (74)     (72)       8     33     10     24     23       7)     (75)     (73)	33         10         24         23           (75)         (73)			r lin., °C) F 23 74) 22 72) 22 72) 23 73)
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	55         21         23         23         23           (74)         (74)         (74)         (74)           53         18         23         23           (74)         (74)         (74)		r lin., °C) ∓ 23 74) 22 72) 23 73) 23 74) 23 74)
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	- 51 12 26 24 (79) (75)	r lin., °C) F 23 74) 22 72) 22 72) 23 73) 23 74) 23 74) 23 74) 23 74) 23 74) 23 74) 23 74) 23 73) 23 74) 23 75 23 76 23 77 23 76 23 77 23 76 23 77 23 76 23 77 24 77 24 77 25 77 25 77 25 77 25 77 25 77 25 77 25 77 25 77 25 77 25 77 25 77 77 77 77 77 77 77 77 77 7

than for Wall L tests because of a higher laboratory relative humidity when Wall S tests were started.

Maximum and minimum laboratory air temperatures obtained during each steady-state test are also listed in Table 5. The laboratory acts as a guard for the metering chamber during tests conducted in CTL's calibrated hot box.

Thermal conductivity of Wall L and thermal resistances of Walls L and S at a specimen mean temperature of 24°C (75°F) were interpolated from measured values. Thermal conductivity of Wall L is 0.27 W/m  $\cdot$  K (1.86 Btu  $\cdot$  in./h  $\cdot$  ft<sup>2</sup>  $\cdot$  °F) at 24°C (75°F). Overall thermal resistances of Walls L and S, respectively, are 0.92 and 0.83 m<sup>2</sup>  $\cdot$  K/W (5.2 and 4.7 h  $\cdot$  ft<sup>2</sup>  $\cdot$  °F/Btu) at 24°C (75°F).

Thermal resistance of Wall S is 10% less than that for Wall L at 24°C (75°F). The normal weight concrete strip of Wall S is 5.8% of the total wall area.

#### Guarded Hot Plate Test Results

Thermal conductivities of specimens made from concrete mixes used to make Walls L and S were measured using a guarded hot plate. Tests were conducted at CTL in accordance with ASTM Test for Steady-State Heat Flux Measurements and Thermal Transmission Properties by Means of the Guarded-Hot-Plate Apparatus (C 177) and ASTM Practice for the Calculation of Thermal Transmission Properties from Steady-State Heat Flux Measurements (C 1045).

Test Specimens—Two specimens were tested from the lightweight concrete for Wall L, the lightweight concrete for Wall S, and the normal weight concrete for Wall S. Nominal specimen dimensions were 50 by 300 by 300 mm (2 by 12 by 12 in.). Specimens were moist-cured at  $23 \pm 1.7^{\circ}C$  (73.4  $\pm 3^{\circ}F$ ) and 100% RH for seven days, then air-dried at  $23 \pm 3^{\circ}F$  (73  $\pm 5^{\circ}F$ ) and 45  $\pm 15\%$  RH. Specimens were ovendried before testing to eliminate effects of moisture migration during testing. Measured specimen dimensions and unit weights are given in Table 6.

Test Procedure-Test specimen temperatures are measured by chromel/alumel thermocou-

TABLE 6-Measured properties of guarded hot plate test specimens.

Specimen		Overall Dimensions, mm (in.)	Average Thickness, mm (in.)	Ovendry Unit Weight, kg/cu m (pct)
Wall L	Тор	310 x 306 (12.2 x 12.1)	50 (1.98)	771 (48.1)
Lightweight Concrete	Bottom	306 x 307 (12.1 x 12.1)	52 (2.03)	750 (46.8)
Wall S	Тор	305 x 305 (12.0 x 12.0)	51 (1.99)	805 (50.2)
Lightweight Concrete	Bottom	305 x 305 (12.0 x 12.0)	50 (1.99)	801 (50.0)
Wall S	Тор	305 x 305 (12.0 x 12.0)	51 (2.00)	2260 (141)
Normal Weight Concrete	Bottom	305 x 305 (12.0 x 12.0)	51 (2.02)	2270 (142)

ples embedded near the specimen surfaces. Thermocouples were placed in previously sawed grooves. Cement paste was used to fill the groove flush with the specimen surface and to secure thermocouples in place. Cement paste was also used to fill small holes in the specimen surface. The cement paste for lightweight concrete specimens had lightweight aggregate fines.

Embedded thermocouples reduce the effects of thermal contact resistance, which is due to the influence of any thin air gap between thermocouple wire and concrete. More information on embedding thermocouple wires and thermal contact resistance is given in Ref 11.

Test Results—Guarded hot plate test results are presented in Fig. 6 for Walls L and S lightweight concrete specimens and Fig. 7 for Wall S normal weight concrete specimens. Thermal conductivity is shown as a function of mean specimen temperature. Thermal conductivity increases with increasing mean temperature for lightweight concrete and decreases with increasing mean temperature for normal weight concrete.

Thermal conductivities at a specimen mean temperature of 24°C (75°F) were interpolated from measured guarded hot plate values. Thermal conductivities for Wall L, Wall S lightweight, and Wall S normal weight specimens, respectively, are 0.21, 0.21, and 1.82 W/m · K (1.43, 1.48, and 12.66 Btu · in./h · ft<sup>2</sup> · °F) at a specimen mean temperature of 24°C (75°F). Average measured thermal conductivity of the lightweight concrete developed for this project is about one ninth that for normal weight concrete.

#### Heat Flux Transducer Test Results

Test Procedures—Two heat flux transducers (HFTs) were mounted on each wall specimen as shown in Figs. 2 and 3 and previously described in the Instrumentation section. Sensors were attached near the center of Wall L and on the lightweight concrete portion of Wall S.

Wall L calibrated hot box test results were used to calibrate the HFTs for Wall S. Heat flow through Wall S as measured by the HFT's was determined in accordance with ASTM Practice for In-Situ Measurement of Heat Flux and Temperature on Building Envelope Components (C 1046).

Test Results—Heat flux transducer test results for the lightweight concrete portion of Wall S are presented in Fig. 6. Results are averages for 16 consecutive hours of testing during steadystate temperature conditions. Data were collected during steady-state calibrated hot box tests.

Results are similar for the heat flux transducers mounted on the climatic chamber and metering chamber sides of the wall.

Thermal conductivity of Wall S lightweight concrete at a mean specimen temperature of 24°C (75°F), interpolated from measured values, is 0.26 W/m  $\cdot$  K (1.8 Btu  $\cdot$  in./h  $\cdot$  ft<sup>2</sup>  $\cdot$  °F).

#### Discussion of Results

Figure 6 presents thermal conductivities of the lightweight concrete measured by the calibrated hot box (ASTM C 976), the guarded hot plate (ASTM C 177), and heat flux transducers (ASTM C 1046). Thermal conductivities from calibrated hot box and HFT measurements are greater than those from guarded hot plate tests because guarded hot plate specimens were ovendried to remove moisture, while the wall specimens were air-dried. An increase in specimen moisture content increases thermal conductivity.

Predicted thermal resistances of Walls L and S are presented in Table 7. Values are calculated using results from guarded hot plate tests on ovendry specimens and measured wall thicknesses. Calculation procedures are from the ASHRAE Handbook - 1985 Fundamentals [10].

The predicted thermal resistance of Wall S is 17% less than that for Wall L. This compares to a 10% decrease in measured thermal resistance for Wall S compared to Wall L. A percent reduction comparison is used because predicted values are based on oven-dried specimens and measured values are based on air-dried specimens.





tm, Wall Mean Temperature, °C

30

20

ũ

Wall L, hot box (air-dry specimen) I Wall L, hot plate (ovendry specimens) D Wall S, hot plate (ovendry specimens)

A Wall S, HFT, climatic side (air-dry specimen) X Wall S, HFT, metering side (air-dry specimen)

¤₿

10

2

1.8

1.6

1.4

1.2

1

0

Measured

Thermal

Conductivity,

Btu.in,/

hr.sq ft.°F

Selven suiden a state i state i se





453

TABLE 7—Predicted	thermal	resistance of	of Walls	L and S.
-------------------	---------	---------------	----------	----------

Layer		R Thermal Resistance sq m•K/W (hr•sq tt•°F/Btu)	ð.
	Walf L	Wall S Ltwt Concrete	Wall S NW Concrete
Outside Air Film	0.03	0.03	0.03
	(0.17)	(0.17)	(0.17)
200 mm Thick Concrete Wall	0.98*	0.97*	0.11*
(8-in.)	(5.59)	(5.49)	(0.64)
Inside Air Film	0.12	0.12	0.12
	(0.68)	(0.68)	(0.68)
Total R	1.13	1.12	0.26
	(6.44)	(6.34)	(1.49)

\* Calculated from guarded hot plate thermal conductivities of ovendry specimens at 24°C (75°F) and measured wall thickness.

Wall S R-value calculated using ASHRAE parallel path method (Ref. 10):

U=(1/1.49)+(6/103)+(1/6.34)+(97/103) =0.188 Btu/hr-sq ft-°F =1.07 W/sq m-K

R=1/U = 5.33 hr-sq ft-°F/Btu =0.94 sg m-K/W

#### **Dynamic Calibrated Hot Box Tests**

Exterior building walls are seldom subjected to steady-state thermal conditions. Outdoor air temperatures and solar effects cause cyclic changes in outdoor surface temperatures. Generally, indoor surface temperatures are relatively constant compared to outdoor surface temperatures.

Dynamic tests are a means of evaluating thermal response under controlled conditions that simulate temperature changes actually encountered in building envelopes. Heat flow through walls as a response to temperature changes is a function of both thermal resistance and thermal storage capacity.

#### **Test Procedures**

The lightweight concrete wall, designated Wall L, was subjected to four dynamic temperature cycles using the CTL calibrated hot box. For these tests, the calibrated hot box metering chamber air temperatures were held constant, while climatic chamber air temperatures were cycled over a pre-determined time versus temperature relationship. The rate of heat flow through a test specimen was determined from hourly averages of data.

Results for one cycle, denoted the NBS Temperature Cycle, are presented. Results for other test cycles are presented in Ref 3. The NBS Test Cycle has been applied to more than 25 walls in previous CTL calibrated hot box studies [12.13]. This periodic cycle is based on a simulated solair<sup>5</sup> cycle used by the National Bureau of Standards in their evaluation of dynamic thermal

<sup>5</sup>Sol-air temperature is that temperature of outdoor air that, in the absence of all radiation exchanges, would give the same rate of heat entry into the surface as would exist with the actual combination of incident solar radiation, radiant energy exchange, and convective heat exchange with outdoor air [10].

performance of an experimental masonry building [14]. It represents a large variation in outdoor temperature over a 24-h period. The mean climatic chamber temperature of the cycle is approximately equal to the mean metering chamber temperature.

A dynamic cycle is repeated until a condition of equilibrium is obtained. Equilibrium conditions were evaluated by consistency of applied temperatures and measured energy response. After an equilibrium condition was reached, the test was continued for a period of three days. Results are based on average readings for three consecutive 24-h cycles.

#### Test Results

Measured temperatures for the NBS Temperature Cycle applied to Wall L are presented in Fig. 8. Climatic chamber air  $(t_c)$ , metering chamber air  $(t_m)$ , climatic surface  $(t_{cs})$ , metering surface  $(t_m)$ , and internal wall  $(t_{md})$  temperatures are average readings of 16 thermocouples placed as described in the Instrumentation section. The average climatic chamber air temperature was 20.2°C (68.3°F). The average metering chamber air temperature was 22.3°C (72.1°F).

Measured heat flow for the NBS Temperature Cycle applied to Wall L is presented in Fig. 9. Heat flow is designated positive when heat flows from the calibrated hot box climatic chamber to the metering chamber. Heat flow determined from calibrated hot box tests (ASTM C 976) is denoted  $q_w$ .









FIG. 9-Measured heat flow for NBS test cycle applied to Wall L.

Heat flow from the heat flux transducer (ASTM C 1046) located on the metering chamber side of the test specimen surface is denoted  $q_{\rm hft}$ . Heat flux transducer data were calibrated using results from steady-state calibrated hot box tests on Wall L.

Heat flow predicted by steady-state analysis is denoted  $q_{ss}$ . Values were calculated on an hourly basis from wall surface temperatures using the equation

$$q_{\rm ss} = (t_{\rm ms} - t_{\rm cs})/R \tag{1}$$

where

 $q_{\rm m}$  = heat flow through wall predicted by steady-state analysis, W/m<sup>2</sup> (Btu/h · ft<sup>2</sup>),

R = average thermal resistance, m<sup>2</sup> · K/W (h · ft<sup>2</sup> · °F/Btu),

 $t_{cs}$  = average temperature of wall surface, climatic chamber side, °C (°F), and

 $t_{ms}$  = average temperature of wall surface, metering chamber side, °C (°F).

Thermal resistances are dependent on wall mean temperature and were derived from steadystate calibrated hot box test results.

Measured heat flow curves, denoted  $q_w$  and  $q_{hft}$ , show significantly reduced and delayed peaks compared to calculated heat flow, denoted  $q_{ss}$ .

#### Thermal Lag

One measure of dynamic thermal performance is thermal lag. Thermal lag is a measure of the response of indoor surface temperatures and heat flow to fluctuations in outdoor air temperatures. Lag is dependent on thermal resistance and heat storage capacity of the test specimen, since both of these factors influence the rate of heat flow.

Calibrated hot box thermal lag is quantified by two methods. In one measure, lag is calculated as the time required for the maximum or minimum specimen surface temperature on the metering chamber side to be reached after the maximum or minimum climatic chamber air temperature is attained. In the second measure, lag is calculated as the time required for the maximum or minimum heat flow rate,  $q_w$ , or  $q_{hft}$ , to be reached after the maximum or minimum heat flow rate based on steady-state predictions,  $q_{ss}$ , is attained. The second measure is illustrated in Fig. 9 for the NBS Test Cycle applied to Wall L. Both measures give similar results. Average thermal lag was 6.5 h for the NBS Test Cycle applied to Wall L. The value determined using heat flux transducer data is the same as that determined from calibrated hot box test results.

Lag times of 3 to 15 h are generally beneficial for exterior walls. Walls with these lag times delay peak afternoon heat loads until cooler night hours. Thermal lags as low as 3 h are beneficial in delaying peak afternoon loads until cooler evening hours. These lower lag times are especially beneficial in commercial and industrial buildings that are vacated in the evening hours. The "lag effect" is also beneficial for passive solar applications.

#### **Reduction in Amplitude**

Reduction in amplitude is a second measure of dynamic thermal performance. Reduction in amplitude, as well as thermal lag, is influenced by both wall thermal resistance and heat storage capacity. Reduction in amplitude is dependent on the temperature cycle applied to the test specimen.

Reduction in amplitude is defined as the percent reduction in peak heat flow when compared to peak heat flow calculated using steady-state theory. Reduction in amplitude is illustrated in Fig. 9. Values for reduction in amplitude were calculated using the equation

$$A = [1 - (q' - \bar{q})/(q'_{ss} - \bar{q}_{ss})] \cdot 100$$
<sup>(2)</sup>

where

A = reduction in amplitude, %,

q' = maximum or minimum heat flow through wall,

 $\overline{q}$  = mean heat flow through wall,

 $q'_{ss}$  = maximum or minimum heat flow through wall predicted by steady-state analysis, and  $\bar{q}_{ss}$  = mean heat flow through wall predicted by steady-state analysis.

Average reduction in amplitude for heat flow measured by the calibrated hot box,  $q_w$ , is 47% for the NBS Temperature Cycle. Reduction in amplitude from heat flux transducer measurements is 58%.

Amplitudes for heat flux transducer data,  $q_{hft}$ , are generally not the same as those for calibrated hot box measurements,  $q_w$  [12,13]. Heat flow amplitudes differ because of the physical presence of the instrument mounted on a wall. A wall's thermal properties are locally altered by the heat flux transducer. In addition, heat flux transducer calibration using steady-state hot box results may not fully correct for dynamic effects of the instrument location.

Actual maximum heat flow through a wall is important in determining the peak energy load for a building envelope. Test results show that anticipated peak energy demands based on actual heat flow are less than those based on steady-state predictions for walls with thermal storage capacity [1]. As expected, calculations based on steady-state analysis overestimate peak heat flow for the dynamic temperature cycle applied to Wall L.

#### Total Heat Flow

Results of dynamic tests are also compared using measures of total heat flow through a specimen for a 24-h temperature cycle. Figure 9 can be used to illustrate total measured heat flow. The curve marked " $q_w$ " is heat flow through the test wall measured by the calibrated hot box. Areas enclosed by the measured heat flow curve and the line for zero heat flow are total heat flow through a wall. The sum of the areas above and below the horizontal axis is total measured heat flow for a 24-h period. A similar procedure is used to calculate total heat flow for a 24-h period from measured heat flux transducer data,  $q_{hft}$ , and predictions based on steady-state analysis,  $q_{ss}$ .

Total heat flows for a 24-h period measured by the calibrated hot box, measured by heat flux transducers, and predicted by steady-state analysis, respectively are 129, 107, and 294 W  $\cdot$  h/m<sup>2</sup> (40.9, 33.8, and 93.3 Btu/ft<sup>2</sup>).

Total heat flow measured by the calibrated hot box is 44% of total heat flow calculated using steady-state analysis. The ratio of total measured heat flow to steady-state predictions, denoted the total heat flow ratio, depends on the climatic chamber air temperature cycle applied to the wall. Particularly for massive walls, greater reductions in actual heat flow, compared to steady-state predictions, occur for temperature cycles which produce heat flow reversals through a wall.

It should be noted that comparison of total measured heat flow values are limited to the specimen and dynamic cycle evaluated in this program. Results are for a particular diurnal test cycle and should not be arbitrarily assumed to represent annual heating and cooling loads. In addition, results are for an individual opaque wall assembly. As such, they are representative of only one component of the building envelope.

#### Comparisons with Other Concrete Walls

Dynamic heat transmission coefficients of thermal lag, reduction in amplitude, and total heat flow ratio are used to compare dynamic thermal response of alternative wall systems.

Thermal lag and reduction in amplitude are dependent on both thermal resistance, R, and heat storage capacity.

ρcL

#### where

 $\rho$  = wall density, kg/m<sup>3</sup> (pcf),

c = wall specific heat, J/kg · K (Btu/lb · °F), and L = wall thickness. m (ft).

A

Mass,  $\rho L$ , is the predominant factor in determining heat storage capacity of most building materials.

For homogeneous walls, thermal lag and reduction in amplitude increase with an increase in M [15]:

$$\mathcal{A} = \left(\frac{L^2/\alpha}{P}\right)^{1/2} = \left(\frac{(R) \cdot (\rho cL)}{P}\right)^{1/2}$$
(3)

where

- L = wall thickness, m (ft),
- $\alpha$  = thermal diffusivity,  $k/\rho c$ , m<sup>2</sup>/s (ft<sup>2</sup>/h),
- k = thermal conductivity of wall, W/m · K (Btu/h · ft · °F),
- $\rho$  = wall density, kg/m<sup>3</sup> (pcf),
- c = wall specific heat,  $J/kg \cdot K$  (Btu/lb · °F).
- R = wall resistance, m<sup>2</sup> · K/W (h · ft<sup>2</sup> · °F/Btu), and
- P = period of dynamic cycle, h.

Table 8 presents values of M and dynamic heat transmission coefficients for Wall L and three other homogeneous concrete walls. Thermal lag, reduction in amplitude, and total heat flow ratio are for the NBS Temperature Cycle applied to each wall using the calibrated hot box. Thermal resistances used in Eq 3 to calculate M are for a wall mean temperature of 24°C (75°F) and are from measurements using CTL's calibrated hot box. Surface resistances are not included in resistances used in Eq 3.

	c pecific Thermal Amplitude Total Marked Mark hr. Heat Row Hast, Utg Reduction, Heat Row Auto-ref. hr. &	810 4.0 45 53 0.75 (0.19)	960 5.5 54 48 1.09 (0.23)	460 6.5 47 44 0.87 (0.11)	750 8.5 61 39 1.20 (0.18)	
Measured Pro	R. Thermal esistance, q m·K/W sq ft·*F/Btu)	0.12 (0.71)	0.31 (1.75)	0.77 (4.4)	1.00 (5.9)	C (75°F)
	Estimated Moisture Content, Ri % ovendry a	5	ø	N	°-	moerative of 24°
	Unit Weight, kg(cu m (pod)	2310	1630 (102)	006 (95)	740 (46)	a wat mean to
	L Thickness, mm (n.)	211 (8.31)	210 (8.28)	203 (8.00)	216 (8.52)	oct roci the at
	Raference	۲	Ø	I	Ø	STM C9761
	Wal Type	Normal weight structural concrete with sand and gravel aggregate	Lightweight structural concrete with expanded shale æggregate	Lightweight structural concrete with Macrolite™ aggregate	Low density concrete with expanded periite aggregate	From calibrated hot hox (A:
	Val	2	3	ب	ទ	•

r roun card area new twitted and twitted next results at a waa mean temperature of 24-1. Vatues do not include surface film resistances.

\* Measured using U.S. Army Corps of Engineers Method CRD-C124-73

CVd

Temperature

Dynamic '

the NBS |

using

Measured

: